| Make |  |
| :--- | :--- |
| Model |  |


| Aristo, Germany |
| :--- |
| Aristo 939, Reinforced Concrete (Stahlbeton) |

Notes

1. This rule is designed for calculating the dimensions of reinforced concrete beams and slabs and the requirements for reinforcing steel.
2. This rule is described as "System Götsch". Nestler also made a reinforced concrete rule which used the system "Jakob".
3. The main scales on the front of the rule are, in effect, the normal A and D on the stock and B and C in the slide. These scales are marked M , for bending moment, b , for breadth of cross-section, r , values for the verification engineer $a n d \mathrm{~h}$, for height. There is also special scale labelled K , intermediate values for calculating area of reinforcing steel.
4. The rule also has a very special cursor with multiple lines, many of them curved. The use of the cursor is explained below.
5. The back of the rule has a number of scales for calculating other dimensions and also a table of weights and sectional areas of standard sizes of steel reinforcing bars.



## Introduction

One of the basic concepts in structural engineering is the "bending moment". It has the dimensions of length times force. In the simple case of a beam supported at both ends carrying a uniform load the maximum bending moment is at the centre of the beam. In this case the bottom of the beam is in tension but the top of the beam is in compression. Concrete is very good at resisting compression but poor at resisting tension. By placing steel bars in those parts of the concrete under tension best use is made of the different properties of these two materials.

Another concept is "shear stress". This is proportional to the rate of change of bending moment and is at its maximum at the end of the beam.

The rule is not specifically designed for calculating the bending moments or shear stress but for calculating suitable dimensions and reinforcement needed.

Whilst a beam, as described above, is a simple case, most structures are more complicated involving a combination of beams, columns and slabs, few of which will be simply supported at the ends.

## Basis of calculations

Most countries had their own "codes of practice" which laid down rules relating to methods to be applied and assumptions which could be made when designing reinforced concrete structures. The normal cursor provided with this rule, which had the reference L939/15, was intended for use with the German code (DIN 1045). This covered a range of tension in the steel from 1200 to 2800 $\mathrm{kg} / \mathrm{cm}^{2}$. The cursor on the rule shown here (which was used by a French structural engineer but which had the instructions in Spanish) has the reference L939/10 and covered the range 880 to $2200 \mathrm{~kg} / \mathrm{cm}^{2}$. A third option had the reference L939/8 and covered the range 880 to $2000 \mathrm{~kg} / \mathrm{cm}^{2}$.

The rule could be used for rectangular beams, with single or double reinforcement, slabs and T-beams. It could be used for bending along an axis, longitudinal forces and shear.

## Dimensions of a beam

This diagram shows a cross section through a beam. Typically, part of the cross-section will be in compression, the upper part on the middle of beam for example, and part in tension. For any given bending moment there is a large number of options for dimensions of the beam and for reinforcement. The normal procedure is to try different combinations of height and breadth for given loads and strengths of materials and select the one which is most economical.

The example below shows the following values:

- The breadth $(25 \mathrm{~cm})$ on the slide is set against the bending moment ( $5200 \mathrm{~kg} . \mathrm{m}$ ) on the upper stock.
- The vertical line on the cursor is then aligned with the height (61 cm ).
- The curved lines on the cursor relate to the tension in steel $\left(\sigma_{\mathrm{e}}=\right.$

Zone of compression


Zone of tension

- These curved lines pass through the compression in the concrete ( $\sigma_{\mathrm{b}}=50 \mathrm{~kg} / \mathrm{cm}^{2}$ ) on the sloping line of the slide.
- The numbers in the centre ( 800 to 1400 and 1600) and top (1800 to 2200 and 2400 to 3500 ) of the cursor have the same meaning as the numbers at the bottom but align with the K scale, K being an intermediate value without specific significance. In this case the value of $1600 \mathrm{~kg} / \mathrm{cm}^{2}$ aligns with a K value of 4.02 .
- The value of $\mathrm{r}, 0.427$ on scale on lower edge of the slide, is used for cross-checking calculations.


A small diagram on the back of the slide gives a summary of the relative position of the different variables.


Cross-sectional area of steel
With this cursor the sectional area of steel is given by:
$\mathrm{F}_{\mathrm{e}}=1.5 * \mathrm{~h}(\mathrm{~cm}) * \mathrm{~b}(\mathrm{~m}) / \mathrm{K}$
In our case $\mathrm{F}_{\mathrm{e}}=1.5 * 61 * .25 / 4.02=5.7 \mathrm{~cm}^{2}$.
With the number 15 cursor the factor of 1.5 in the above equation become 1.0 and with the number 8 cursor the factor becomes 1.875.

A table on the back of the slide gives the sectional area of different numbers of reinforcing bars of different standard diameters.

| $\emptyset$ | Gewicht | Umfang | Querschnitt in $\mathrm{cm}^{2}$ für Stückzaht: |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| mm | $\mathrm{kg} / \mathrm{m}$ | cm | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| 8 | 0.395 | 2.51 | 0.50 | 1.01 | 1.51 | 2.01 | ${ }^{2} .52$ | ${ }^{3.02}$ | 3.52 | 4.02 | 4.53 | 5.04 | 5.54 | 6.04 |
| 10 | 0.617 | 3.14 | 0.79 | 1.57 | 2.36 | 3.16 | 3.93 | 4.71 | 5.50 | 6.28 | 7.07 | 7.86 | 8.64 | 9.42 |
| 12 | 0.888 | 3.77 | 1.13 | 2.26 | 3.39 | 4.52 | 5.65 | 6.78 | 7.91 | 9.04 | 10.17 | 11.30 | 12.43 | 13.56 |
| 14 | 1.208 | 4.40 | 1.54 | 3.08 | 4.62 | 6.16 | 7.70 | 9.24 | 10.78 | 12.32 | 13.86 | 15.40 | 16.94 | 18.48 |
| 16 | ${ }^{1.578}$ | 5.03 | 2.01 | 4.02 | 6.03 | 8.04 | ${ }^{10.05}$ | 12.06 | 14.07 | 16.08 | 18.09 | 20.10 | 22.11 | 24.12 |
| 18 | 1.998 | 5.65 | 2.54 | 5.08 | 7.62 | 10.16 | 12.70 | 15.24 | 17.78 | 20.32 | 22.86 | 25.40 | 27.94 | 30.48 |
| 20 | 2.466 | 6.28 | 3.14 | 6.28 | 9.42 | 12.56 | 15.70 | 18.84 | 21.98 | 25.12 | ${ }_{28.26}$ | ${ }_{31.40}$ | 34.54 | 37.68 |
| 22 | 2.984 | 6.91 | 3.80 | 7.60 | 11.40 | 15.20 | 19.00 | 22.80 | 26.60 | 30.40 | 34.20 | 38.00 | 41.80 | 45.60 |

From this table we can see that two bars of 20 mm diameter would give enough steel and also that a 20 mm bar would weight 2.477 $\mathrm{kg} / \mathrm{cm}$.

The same calculation can be performed on the front of the rule using extra hairlines on the cursor. Setting the small hair line at the bottom right against the diameter $(20 \mathrm{~mm})$ gives the sectional area $\left(3.14 \mathrm{~cm}^{2}\right)$ against the principal hairline on the stock. One can multiply this area by the number of bars to give the total sectional area, in this case $6.28 \mathrm{~cm}^{2}$. From the area of the bar $\left(3.14 \mathrm{~cm}^{2}\right)$ we can use the second small hair line to get the weight per metre, 2.47 kg .


It would be unusual to have just two fairly thick bars in a beam of 25 cm width; the above number was chosen for convenience of presentation.

Calculation of $x$
The distance x , (see above diagram) is the depth of the zone of compression.


The back of the rule has a special scale for this. There is a scale marked $x$ on the stock and special marks on the |l|lili!llilislide, with the black dot with a line through it. This line is brought against the value $K$, in our case 4.02, previously found.

There are other scales by which other special dimensions can be obtained in a similar way.

